



# The “Other” HMA Test

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**101<sup>st</sup> THE**

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**Engineer of Tests**

**Illinois Department of Transportation**



# The “Other” HMA Test for *Flexible* Pavements

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# **special acknowledgements to:**

I. L. Al-Qadi, H. Ozer, J. Lambros, & D. L. Lippert  
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& Aaron Coenen, Greg Renshaw, and Jim Meister

& Jim Trepanier

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**Illinois Center for Transportation  
University of Illinois at Urbana-Champaign**

**ICT - Testing Protocols to Ensure Mix Performance w/  
High RAP and RAS**



8.10.2001



8.10.2001



# HMA Pavement Failures

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- **Rutting**





# Solution – a Performance Test





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Does this test duplicate traffic and conditions?



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- It has steel wheels, not rubber!



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- It takes less than a day .... to simulate 15 years.....



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- It's not based on a fundamental property.



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- The wheels go forward .... and backward on the sample.
- The test occurs in a 50° C water bath.
- It takes less than a day .... to simulate 15 years.....
- It's not based on a fundamental property.
- And ....



# Solution – a Performance Test

It's an Accurate Indicator, Not a Flawed Simulator







# HMA Pavement Failures

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- Rutting
- **Cracking**

# HMA Pavement Failures

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- Rutting
- Cracking
  - Reflective





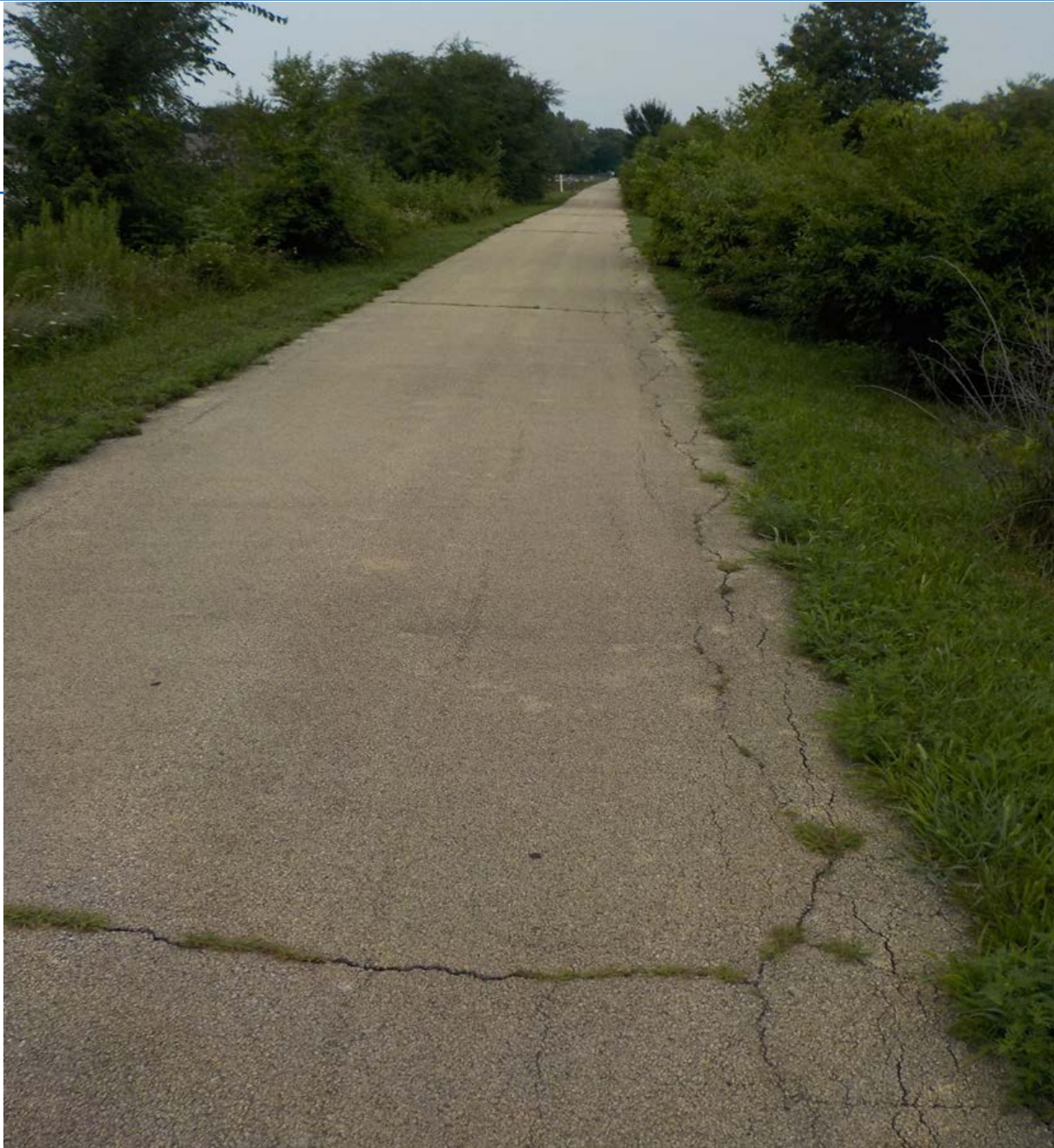


# HMA Pavement Failures

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- Rutting
- Cracking
  - Reflective
  - Thermal (Cold Weather)





# HMA Pavement Failures

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- Rutting
- Cracking
  - Reflective
  - Thermal (Cold Weather)
  - Fatigue







# Where Do We Need to Focus?

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# Where Do We Need to Focus?

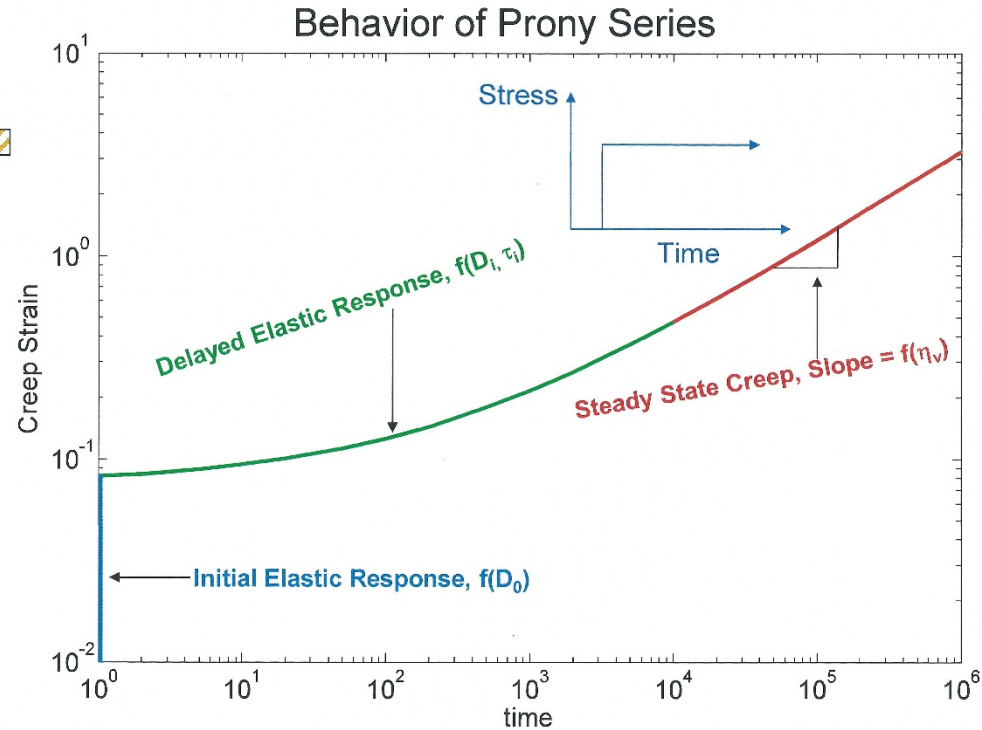
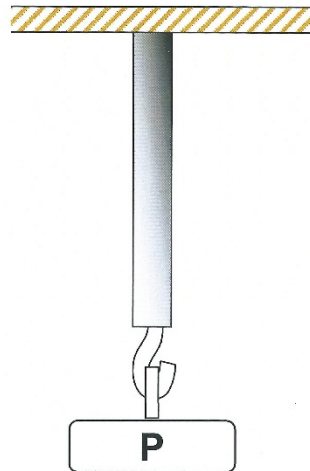
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- **Asphalt Binder**



# 2. Elastic vs. Viscoelastic Response

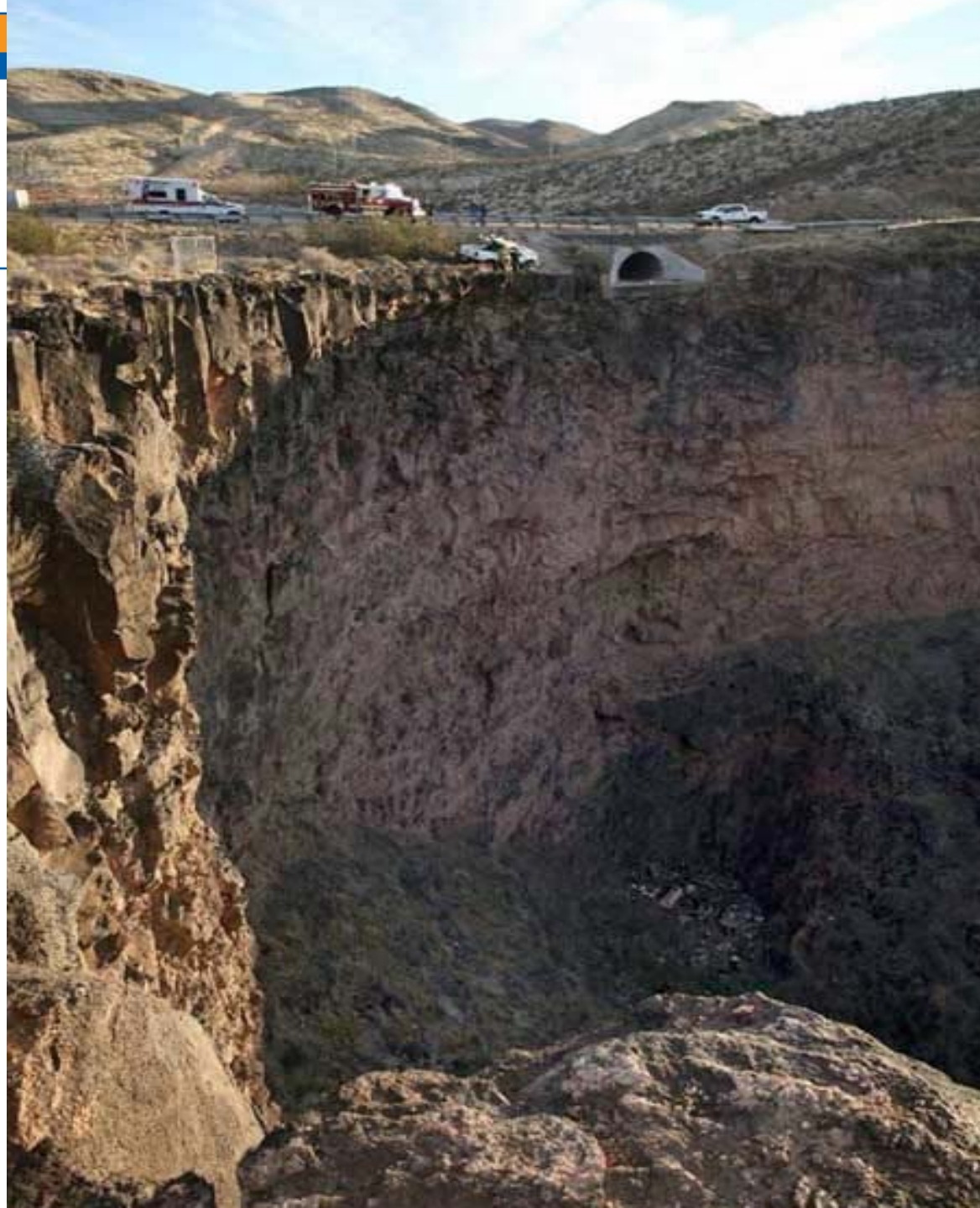
## 2.2. Material Models

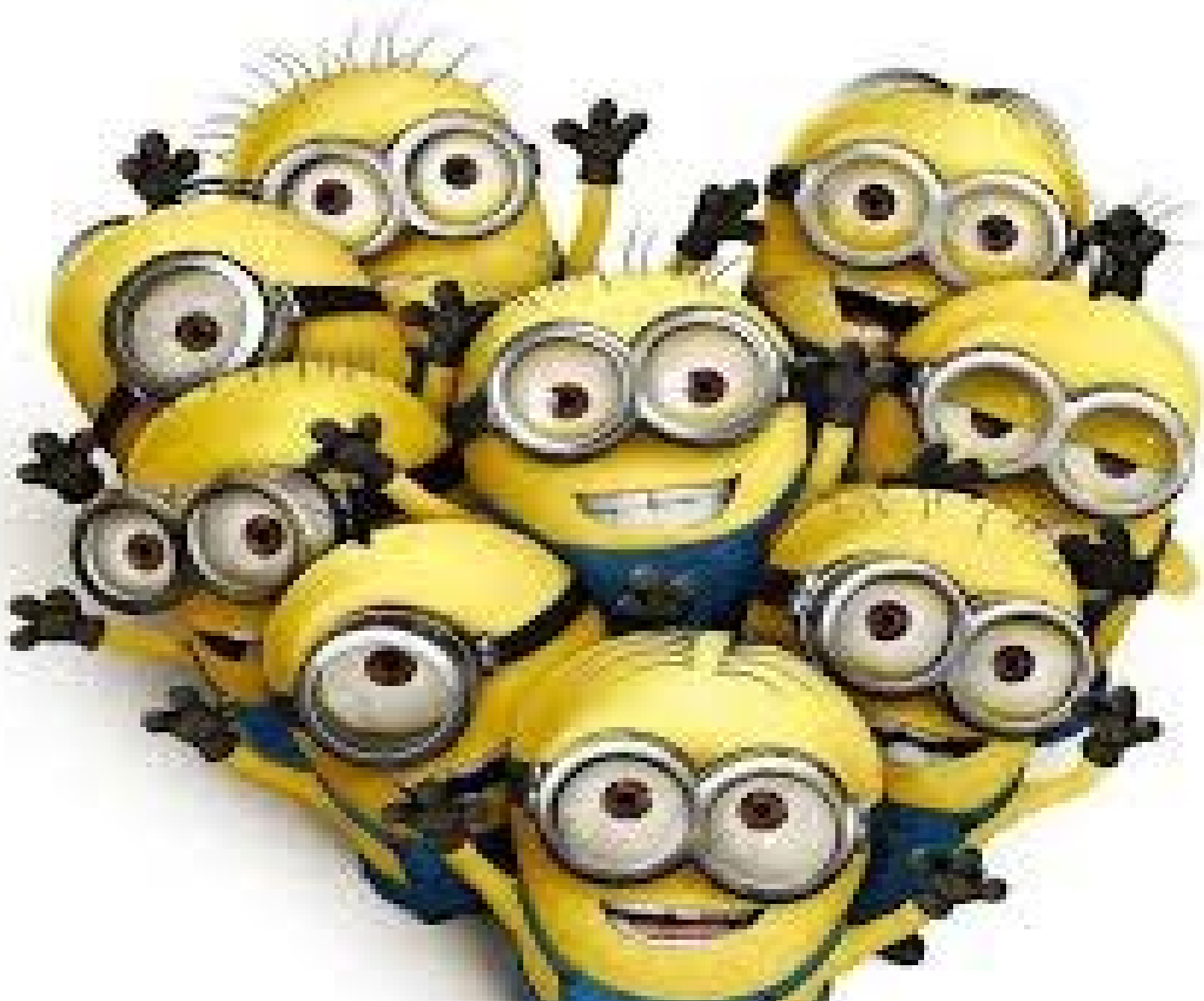


Transportation Research Board Webinar Organized by:  
AFK 50(1): Sub committee on advanced models to understand behavior and performance of asphalt mixtures  
AFK 50: Committee on characteristics of asphalt paving mixtures to meet structural requirements









# Could There be a Single Solution?



# Challenges

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- ❑ SuperPave was developed for **neat materials**
- ❑ More recycled materials are being used in HMA – **less virgin components** – especially PG asphalts in the final mix
- ❑ Currently, some recycled materials are allowed by method specifications intended to limit the risk of cracking by ABR limits and grade bumping, **not actual mix performance**
- ❑ **Fatigue** cracking issue: **stiffer** mixes with high ABR may exhibit early fatigue cracking
- ❑ **Thermal/Block** cracking issue: **stiffer** mixes have **reduced relaxation potential**



# Challenges (RAP/RAS)

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- RAP AC can be hard or soft – depends on project(s) milled
- RAP aggregates may be siliceous or carbonate
- Shingle asphalt (*\*PG 112+02*) is much harder than paving grades
- Counteracting various hard recycled binders with virgin PG binder becomes **arbitrary**
- Neat asphalt **blending** with RAP and RAS for final mix is not understood

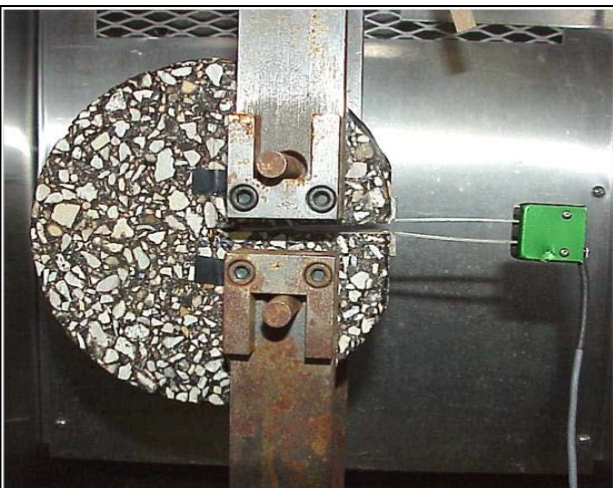


# Test Method Selection Criteria

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- **Practical \$\$**
- **Quick turnaround**
- **Correlation to independent tests and engineering intuition**
- **Significant and meaningful spread in test output**
- **Correlation to field performance**

# Mixture Tests Available



# Mixture Tests Available





# Mixture Tests Available



# Semi-Circular Bending Test



- Relies on simple three point bending
- Easy specimen preparation
- Can use AASHTO T283 equipment \*
- Repeatable



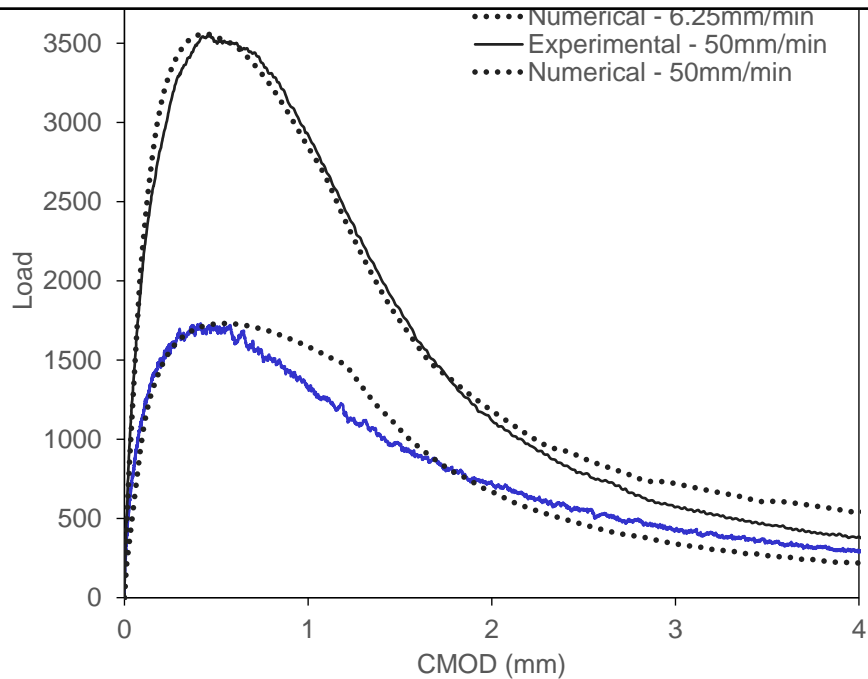
# Research Approach

Parameter	Variables
Material Source	Plant Mixes, Lab-Mixes, Field Cores
N-Design	N30, N50, N70, N80, N90
Nominal Maximum Aggregate Size	4.75 mm, 9.5 mm, 12.5 mm, 19.0 mm
Asphalt Binder	PG52-28, PG58-22, PG58-28, PG64-22, PG70-22, PG70-28, PG76-22
Recycled Materials	RAP, RAS, Recycled Concrete, and Steel Slag
Asphalt Binder Ratio	0 to 60
RAP Content (%)	0 to 53
RAS Content (%)	0 to 8.5

- **Assessment of variety of plant mixes, lab designed mixes, and field cores**
- **Correlation to other tests (modulus and fatigue)**
- **Theoretical and numerical evaluation**

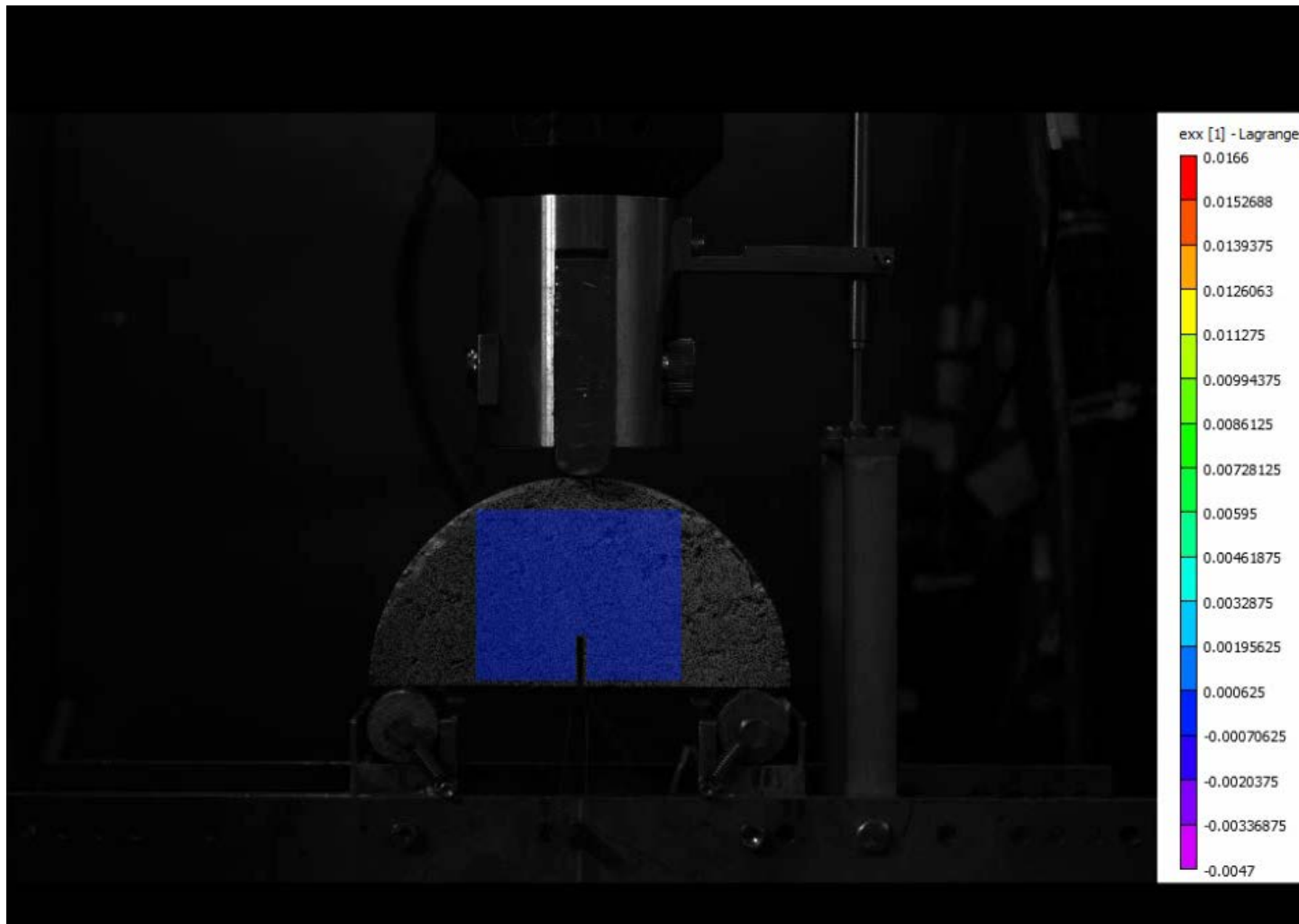
# FEM Results

- FEM simulations of N80-25 mix





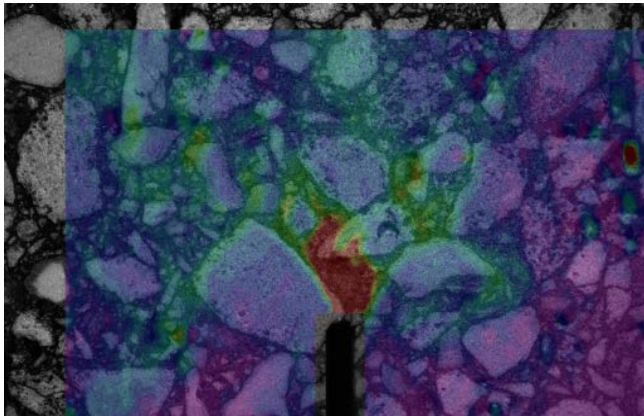
# Fracture Process Zone



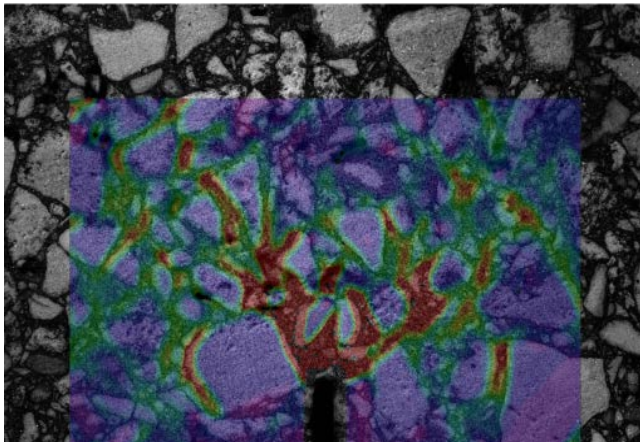
# Fracture Process Zone

## N90 Control (0% RAS)

**-12°C @ 0.7 mm/min**

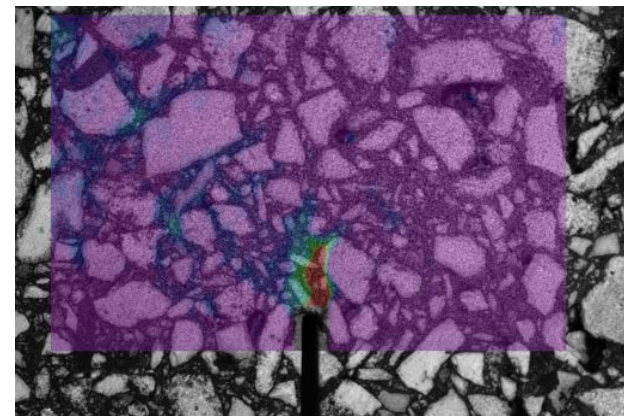


**25°C @ 50mm/min**

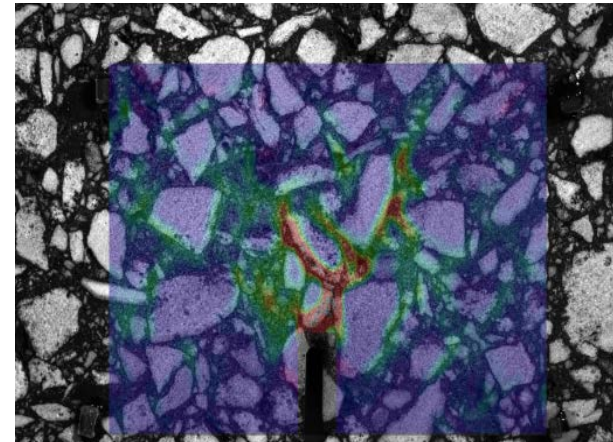


## N90 30% ABR (7% RAS)

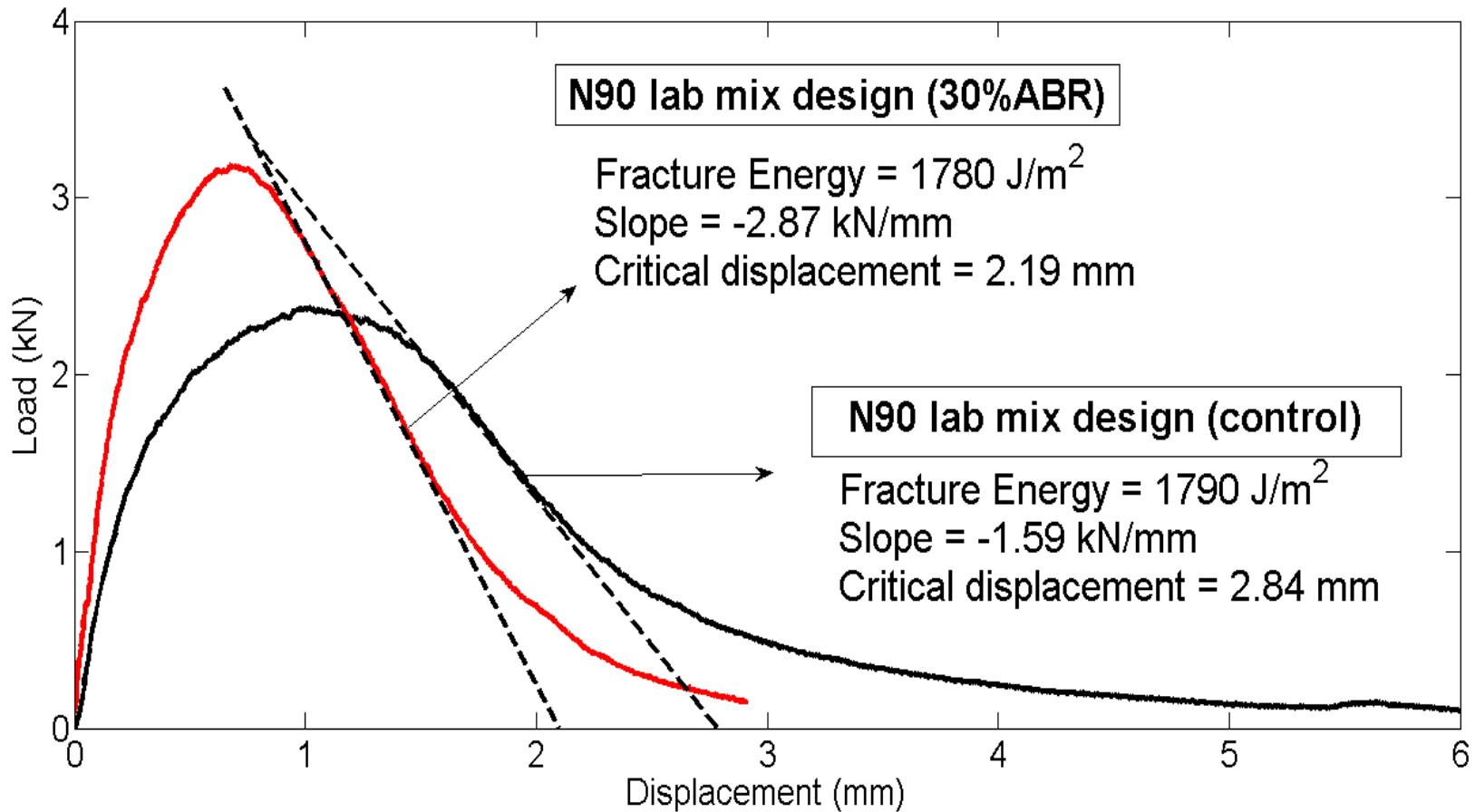
**-12°C @ 0.7 mm/min**



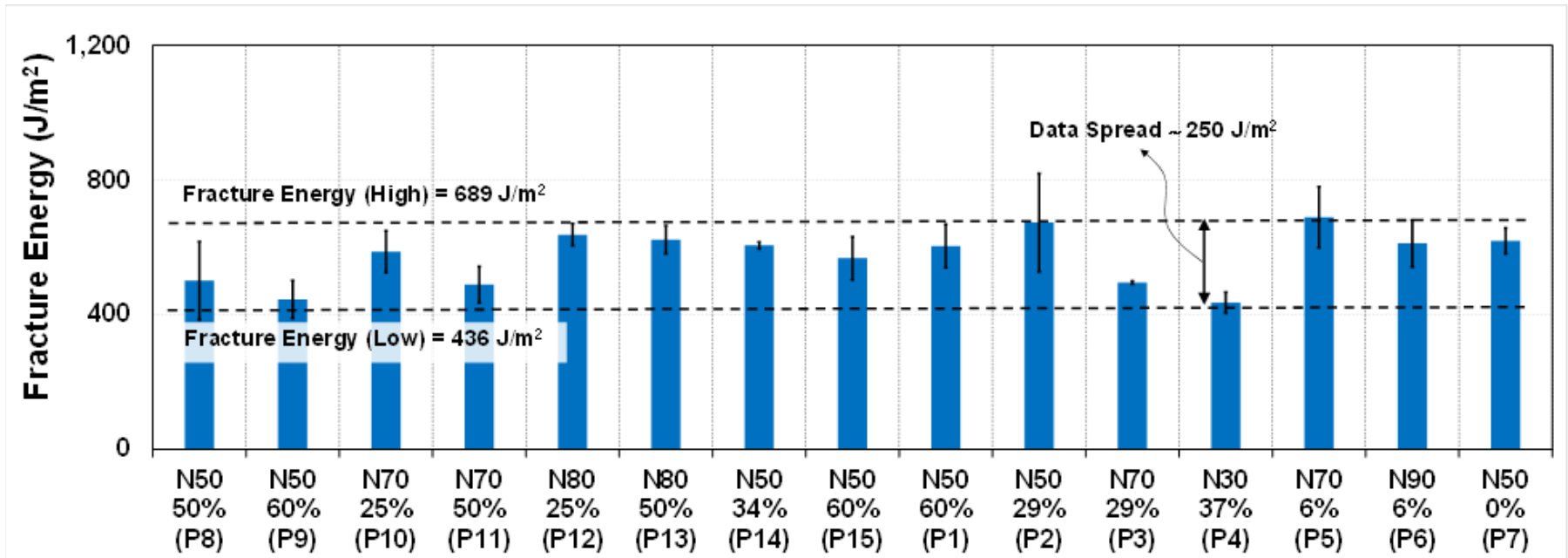
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# SCB Fracture Results



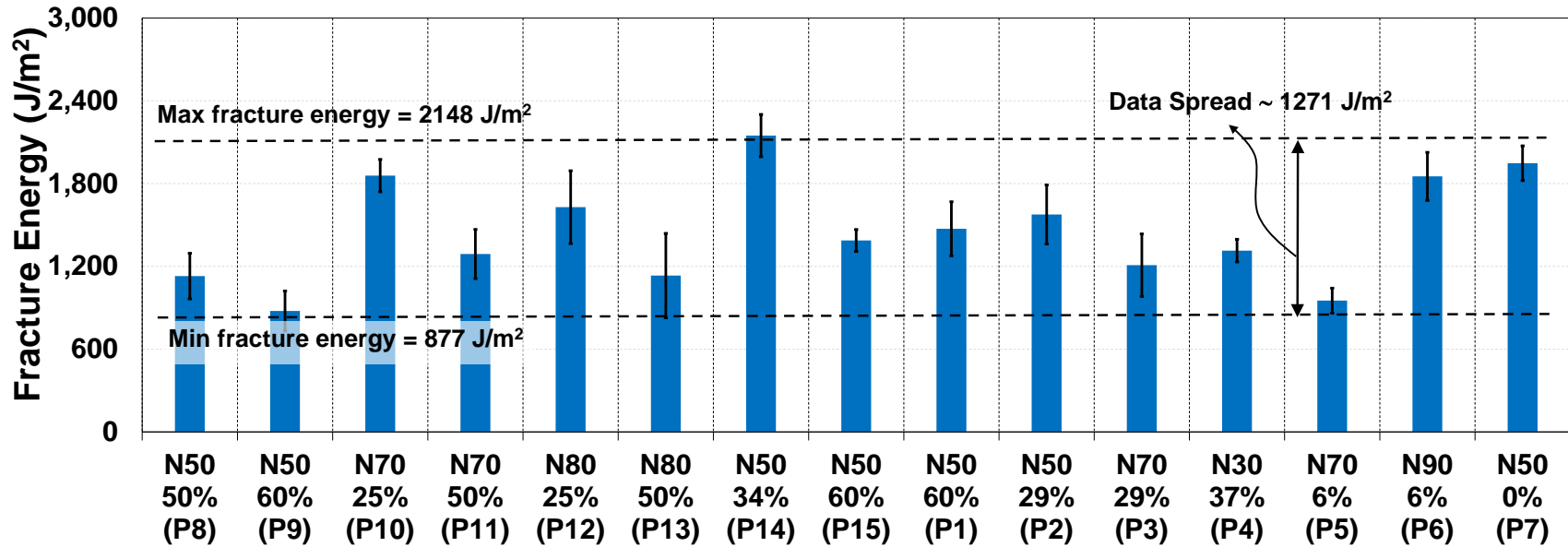
# Establishment of Test Temperature and Loading Rate



- SCB fracture test results at -12°C
- Limited data spread

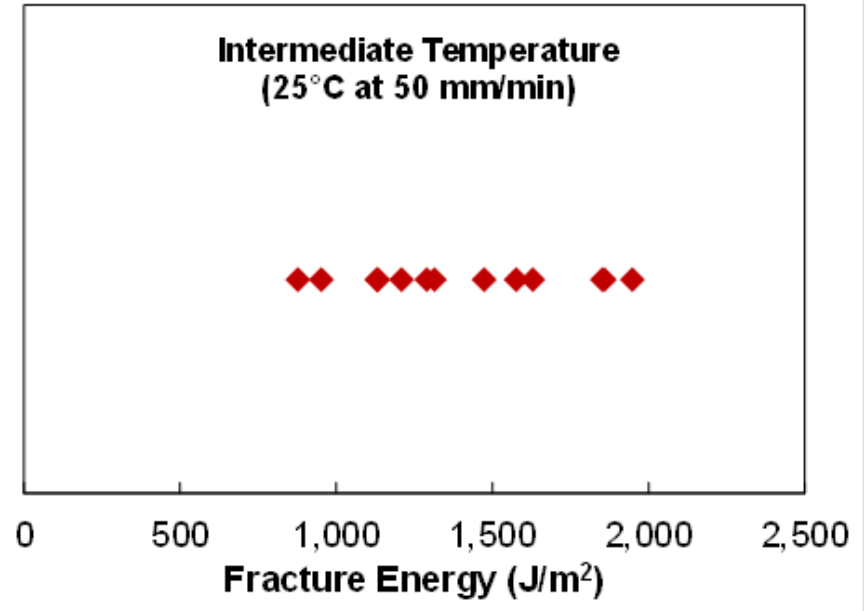
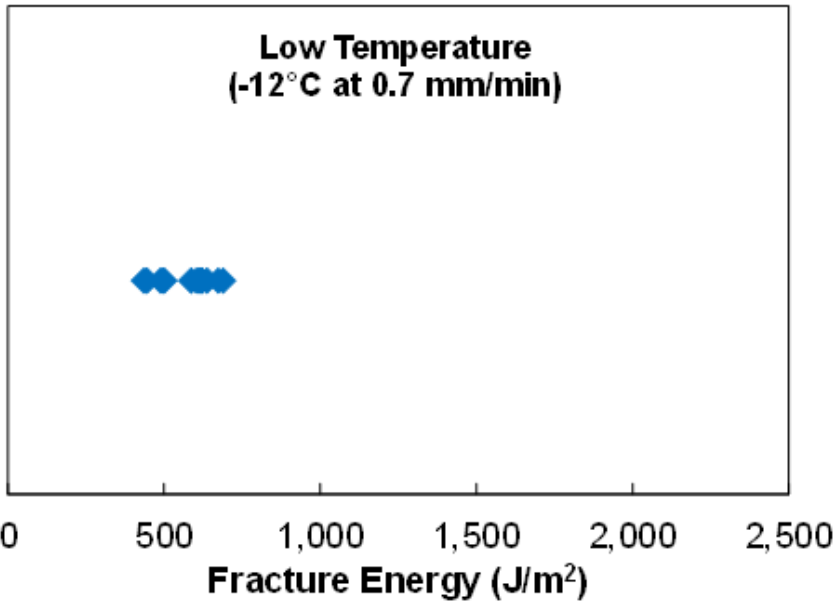


# Establishment of Test Temperature and Loading Rate



- SCB fracture energy results for the same mixes at 25 °C using displacement control at 50 mm/min
- Significant spread in fracture energy

# Establishment of Test Temperature and Load Rate

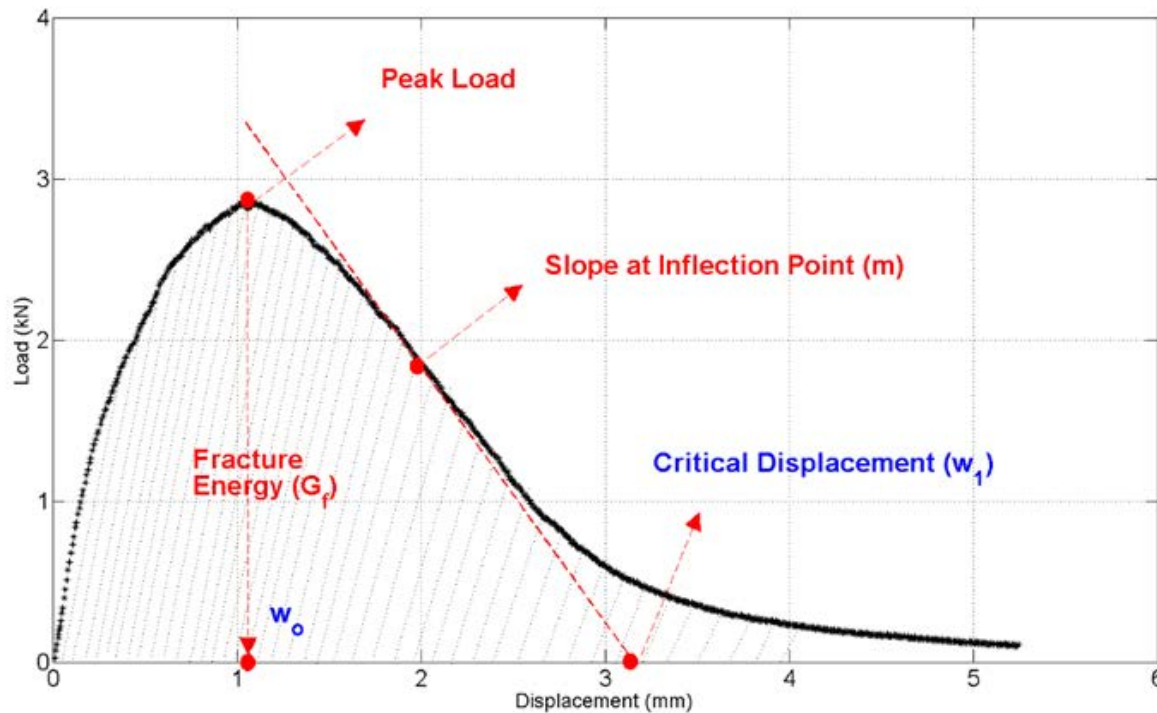


- A comparison of low temperature and intermediate temperature (25°C ) SCB test results indicate the suitability test to discriminate mixes
- 25 °C and 50 mm/min loading rate were selected

# SCB Fracture Results

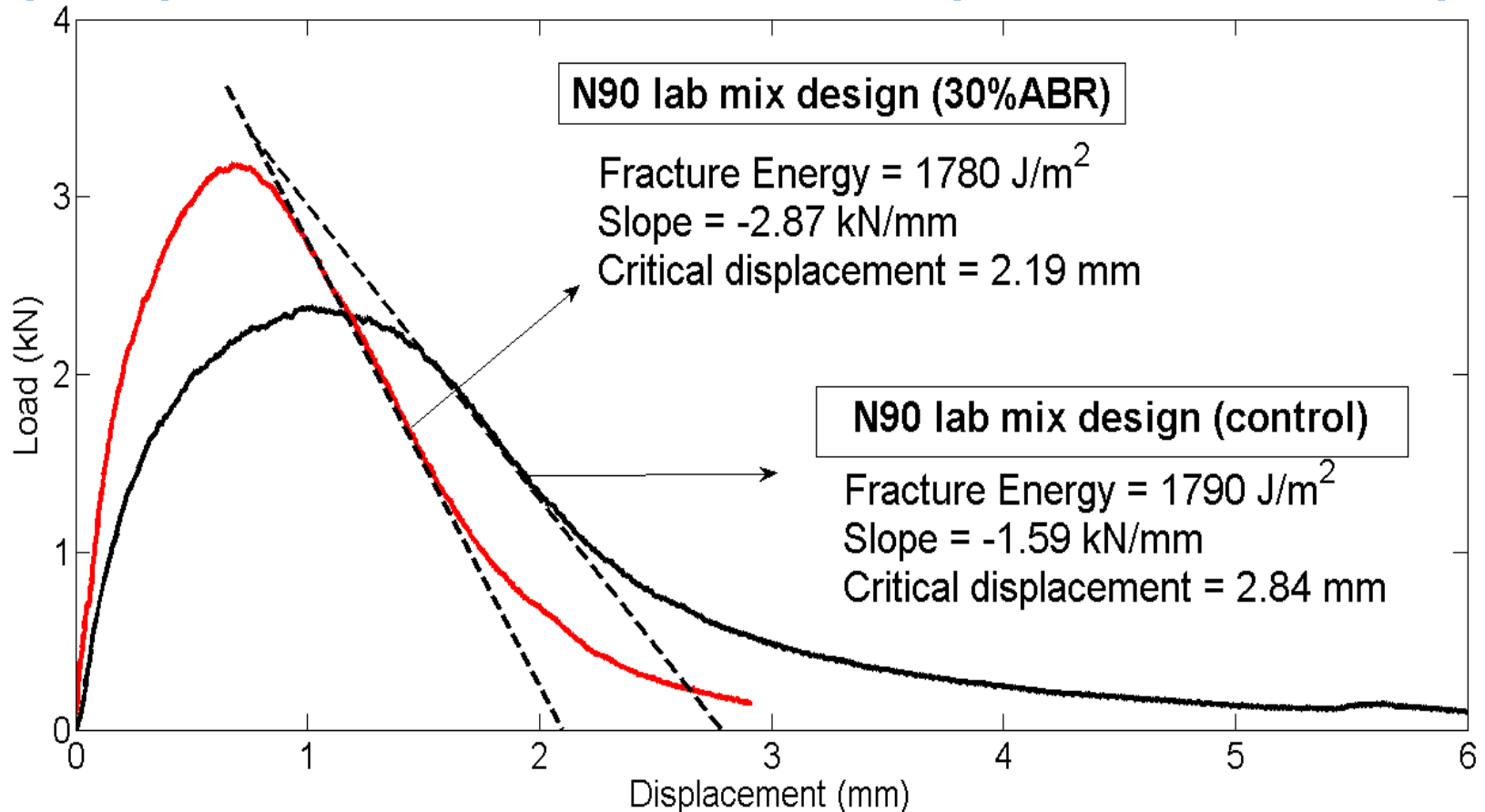
- Flexibility Index calculated for two lab design (N90) mixes w/ and w/o ABR (30% ~ 7% RAS):

$$\text{Flexibility Index (FI)} = A * G_F / m$$



# SCB Fracture Results

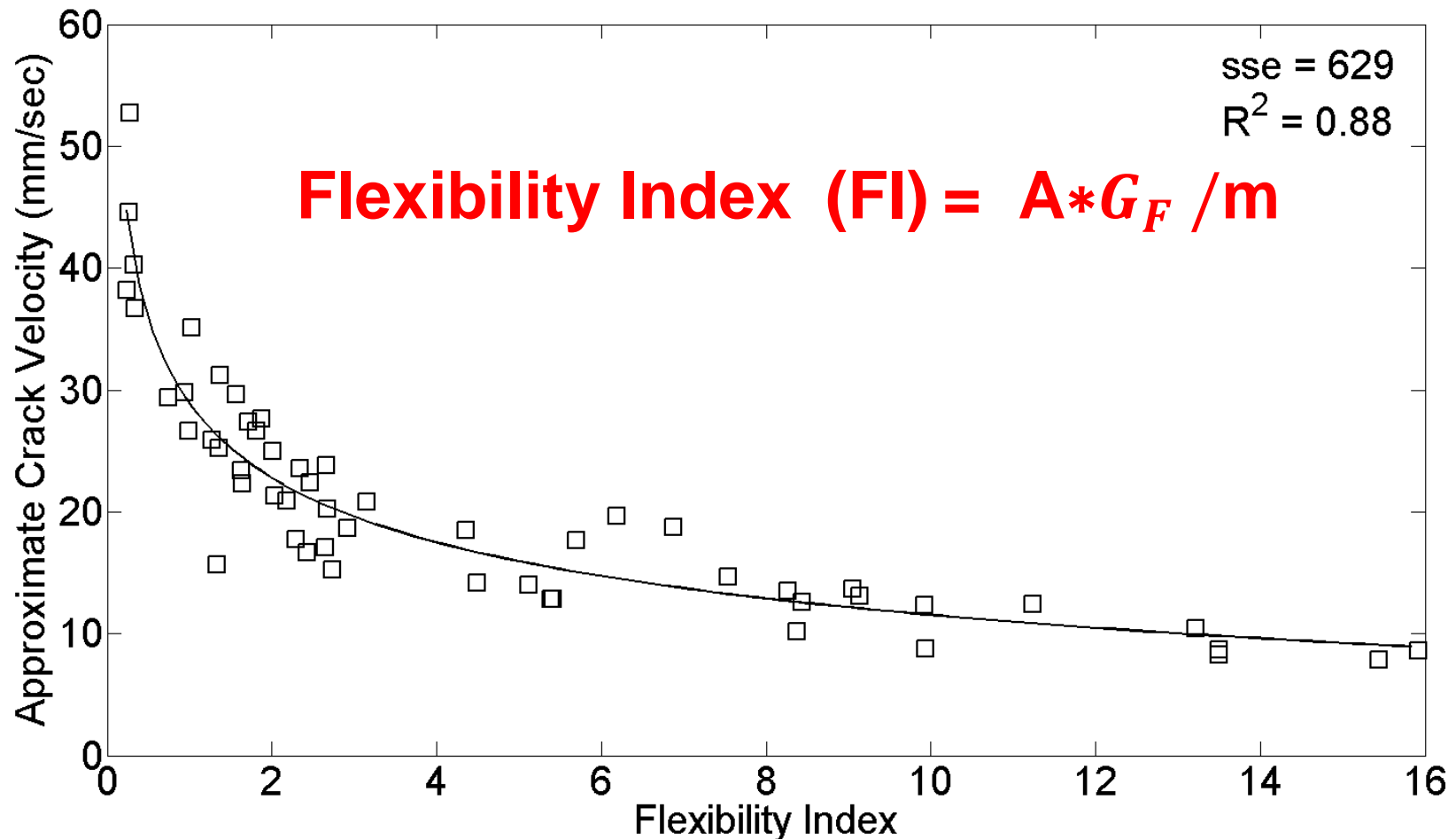
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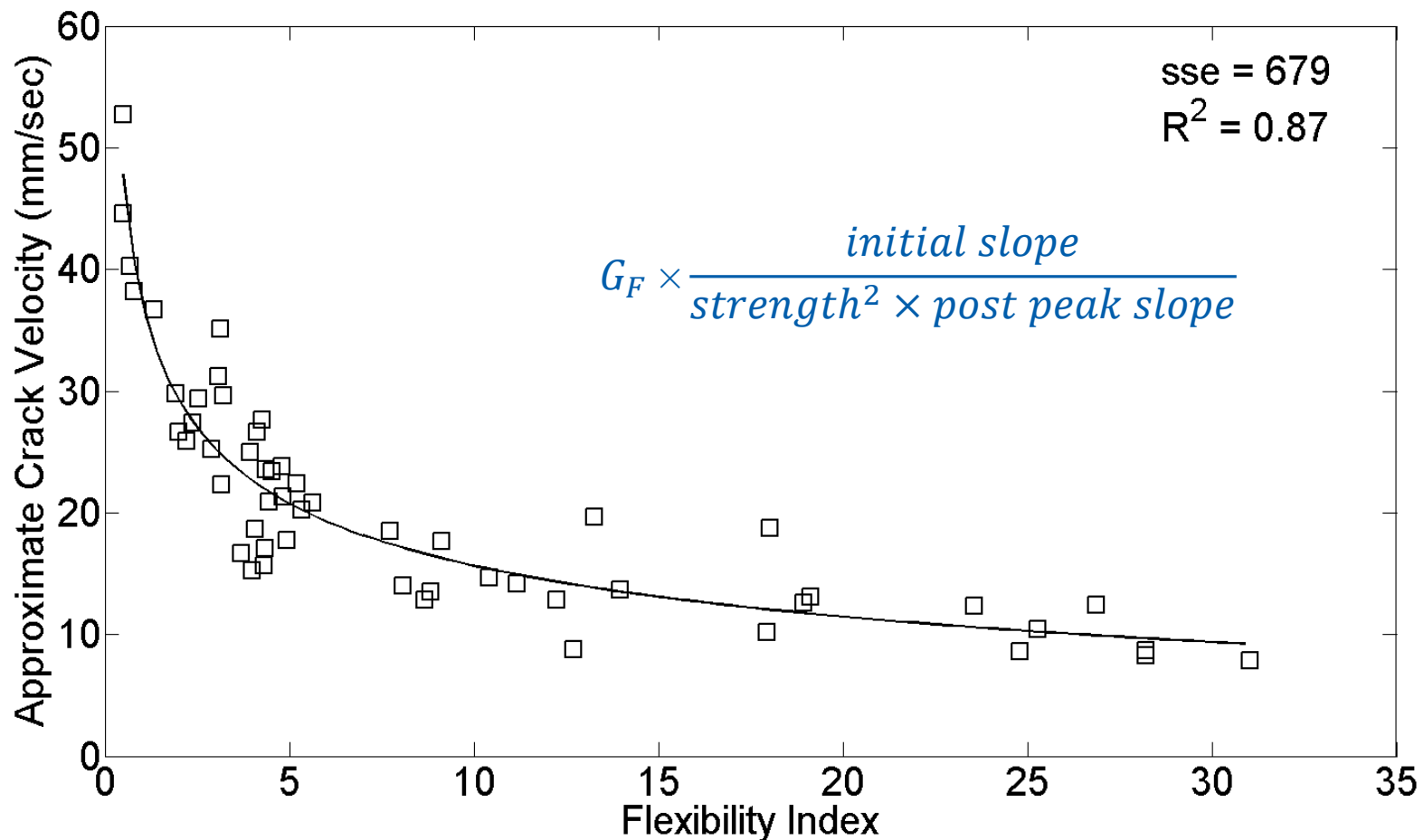
# Development of Flexibility Index

- A **theoretically**-supported flexibility index (FI)



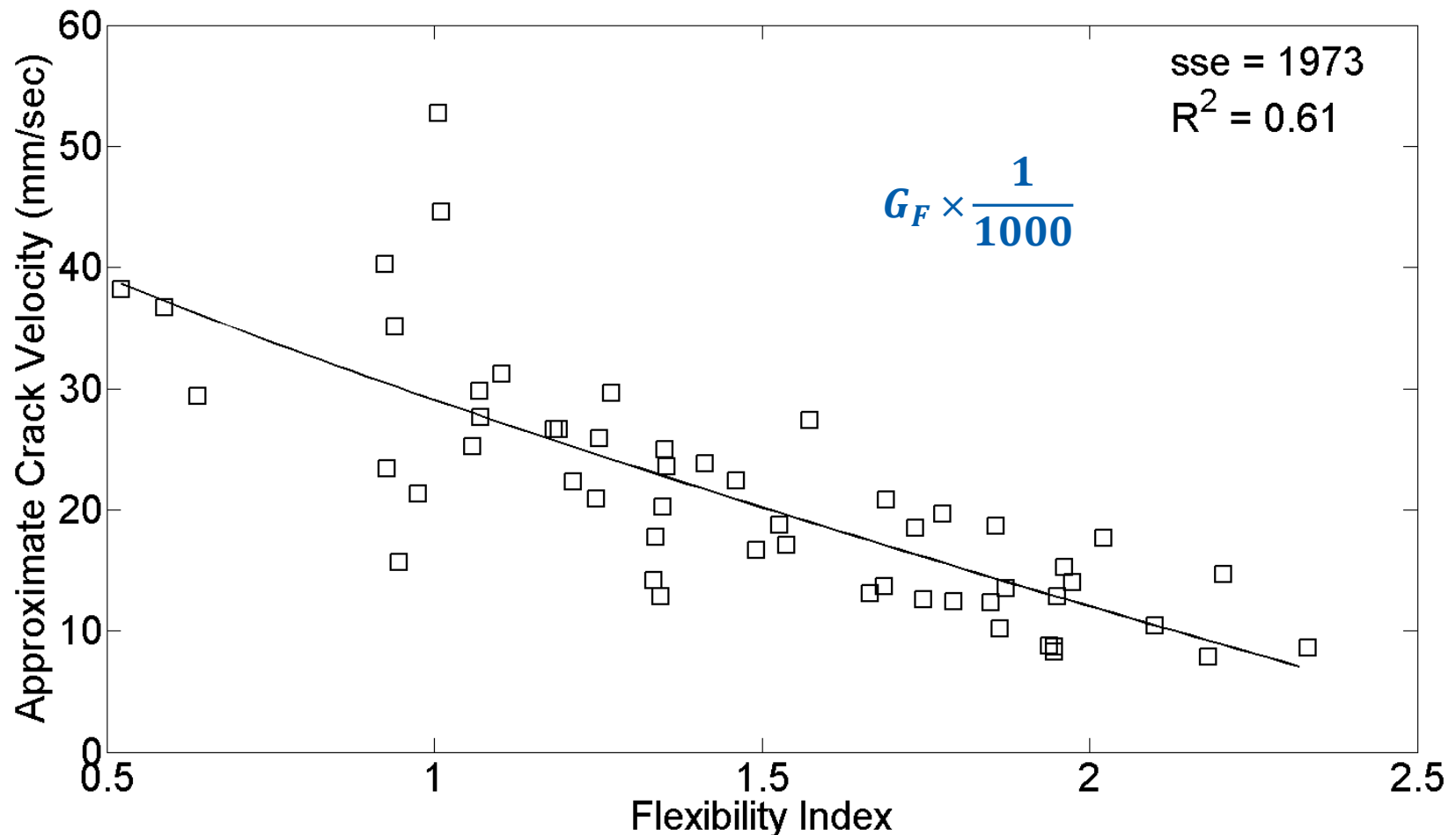
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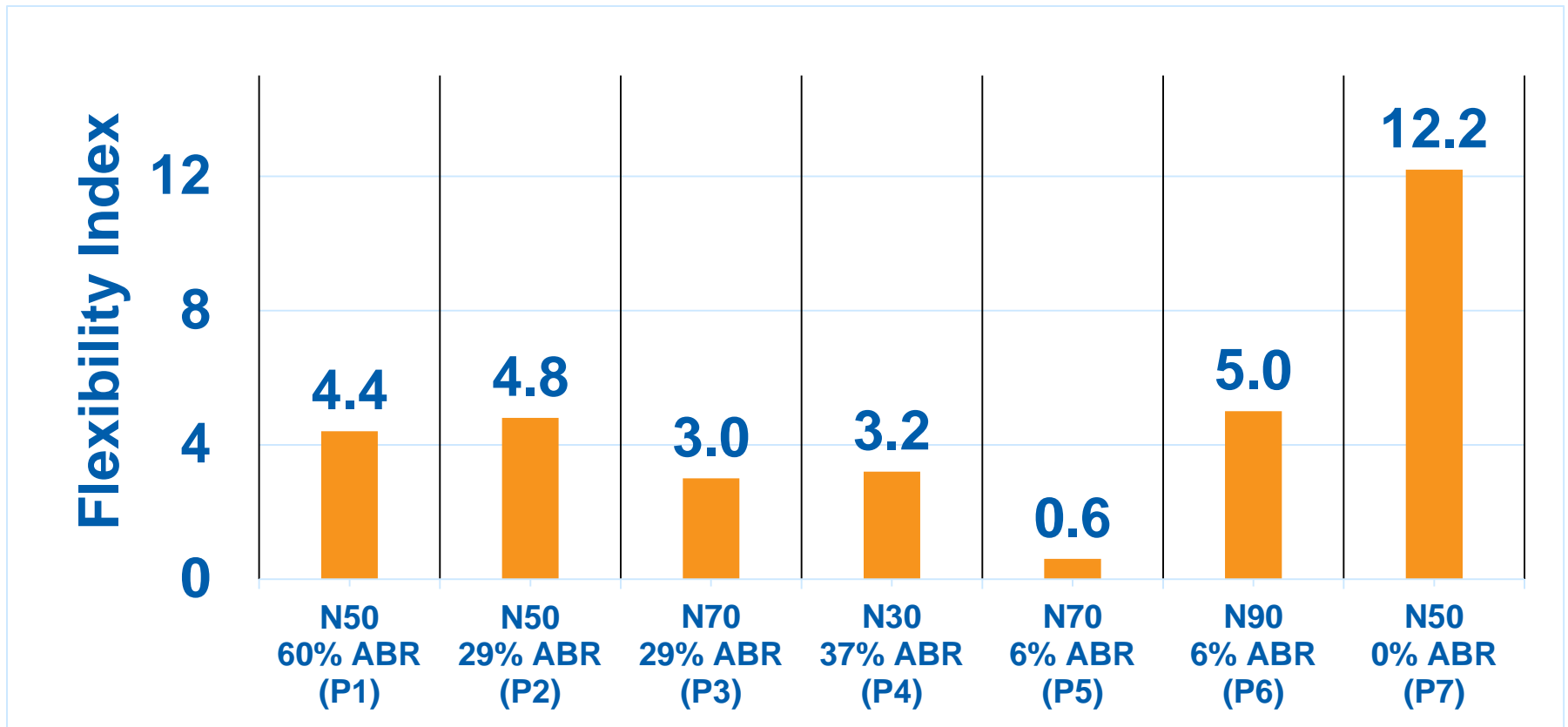
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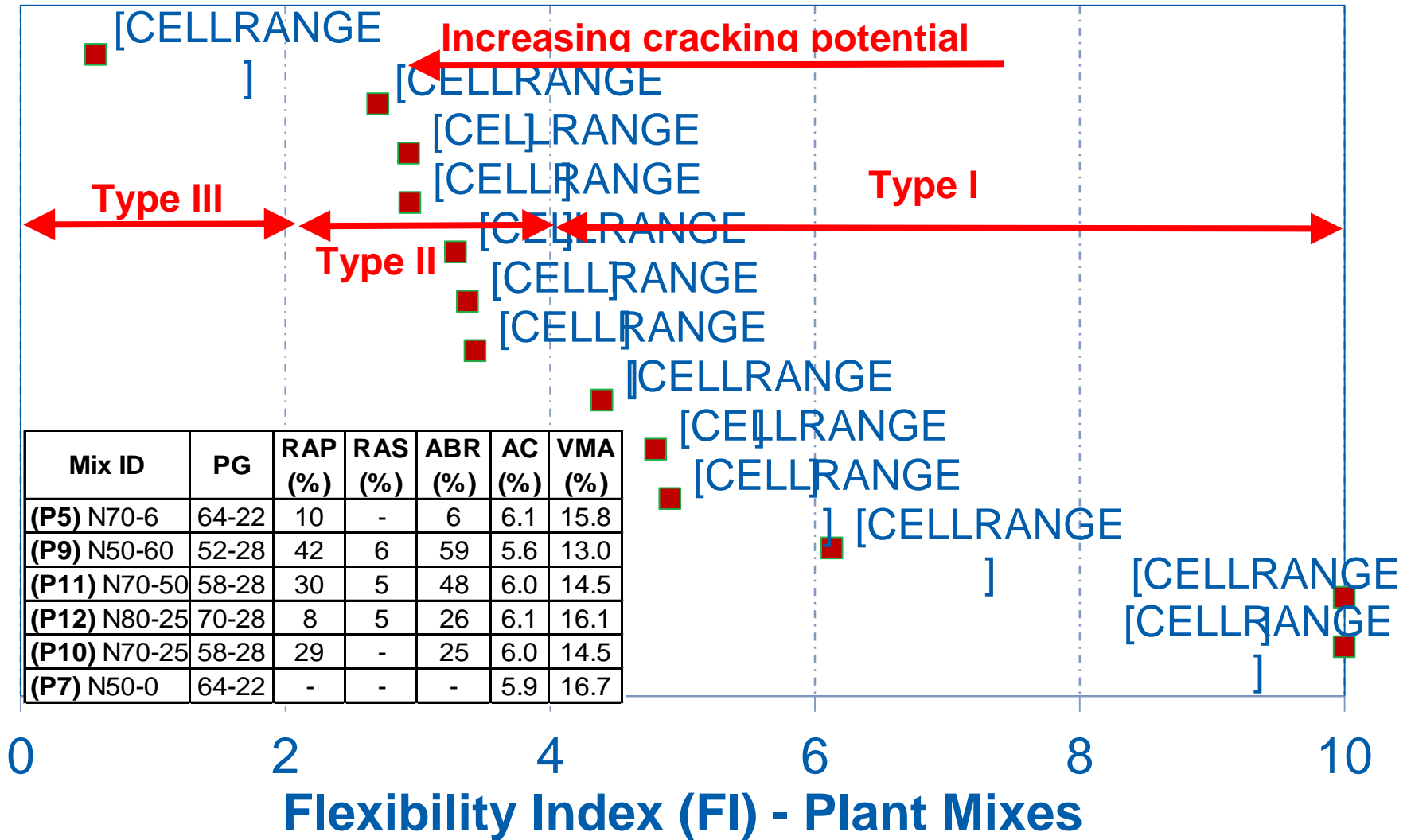
# FI Results

- Flexibility index calculated for selected plant mixes

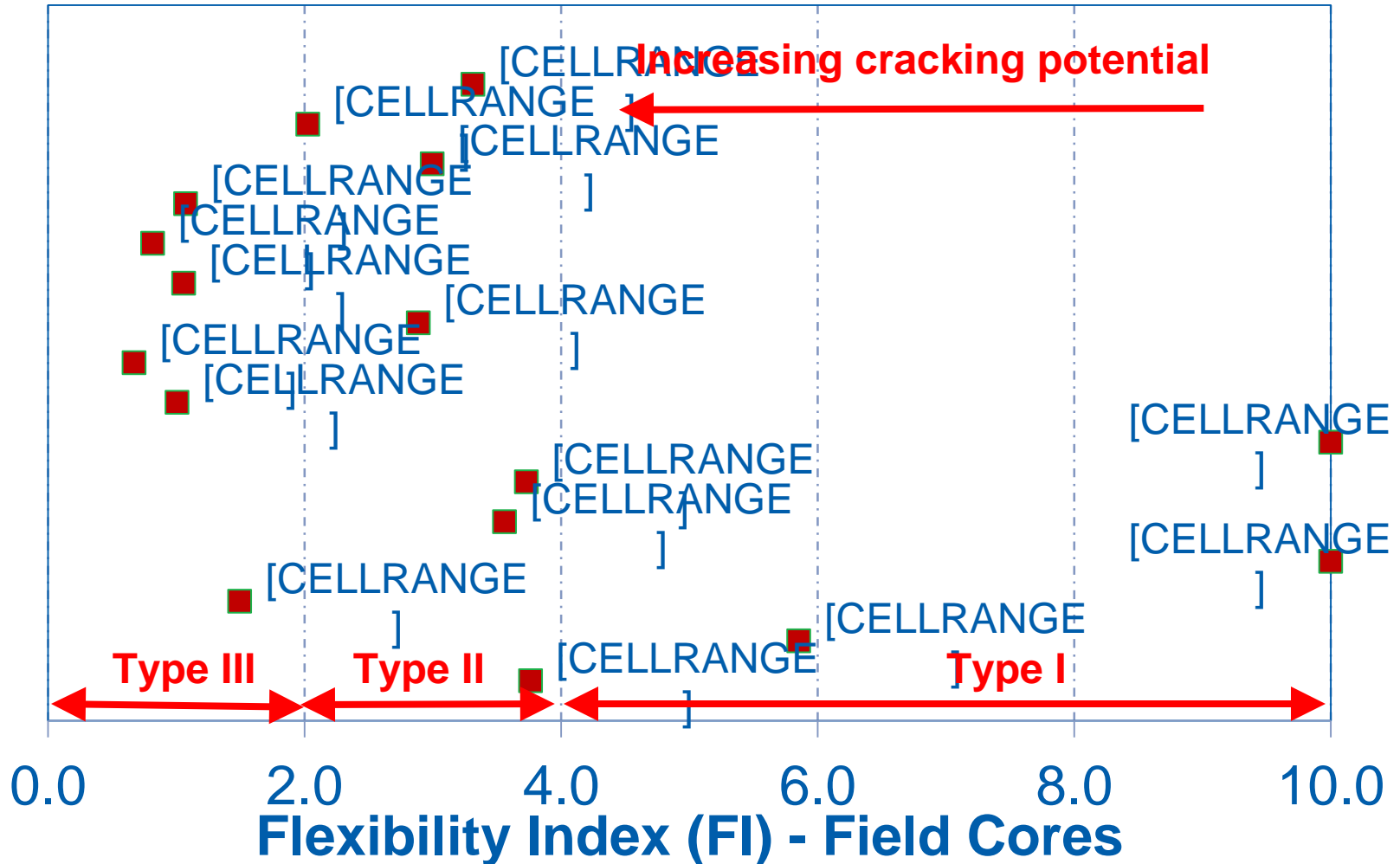




# FI - Plant Mixes



# FI (with SF): Field Cores

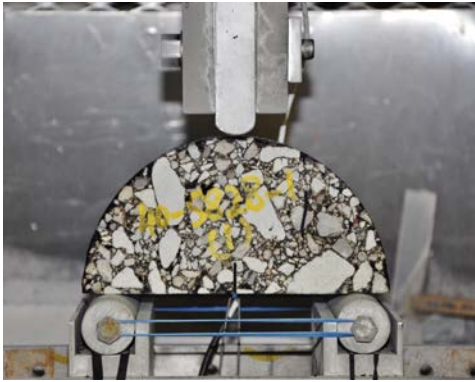


# FI Categorization & Implementation

## □ Draft Categorization of Mixes Using Flexibility Index and Threshold

Mix Category	Mix Type Based on Flexibility Index (FI)	Potential Actions and Remedies
Unacceptable Mix	Type III (<2.0)	Reject mix due to high early cracking potential. Redesign the mix.
Inferior Mix	Type II ( $\leq 2.0-4.0$ )	Mix susceptible to cracking. Use the mix only in temporary application or redesign.
Acceptable Mix	Type I ( $\leq 4.0-10.0^1$ )	Accept the mix. Mix is expected to perform adequately. Use the mix in surface overlay or typical pavement applications.

\*Lab-compacted mix having FI > 10 is considered high performance mix.



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**Low Temperature  
Cracking**

**Fatigue Cracking/  
Service Temperature**

**Permanent  
Deformation**



**-40°C**

**-20°C**

**20°C**

**40°C**

**Low in-service  
temperatures**

**Intermediate in-service  
temperatures**

**High  
Temperatures**





# Owner Concerns

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- ❑ **We don't know where asphalts originate**
- ❑ **We don't know what is added to asphalts**
- ❑ **We don't know what is in recycled materials**
- ❑ **We don't know what happens when sources of asphalt and aggregate change**
- ❑ **We don't know what damage occurs during production in various plants**
- ❑ **We need a mix cracking performance test**



# The Other HMA Test

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# The Other HMA *Performance* Test

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# The Other HMA *Performance* Test

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- With the Hamburg Wheel to minimize rutting probability ....



# The Other HMA *Performance* Test

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- With the Hamburg Wheel to minimize rutting probability ....
- **The SCB reduces risk to the owner of premature pavement cracking**
  - It is simple and scientifically sound
  - Can test gyratory specimens or field cores
  - The **Flexibility Index** can discriminate between good and poor performing mix
  - More validation is underway\*

